

Topological and Hydraulic Metrics-Based Search Space Reduction for Optimal Re-Sizing of Water Distribution Networks

Kegong Diao¹, Luigi Berardi², Daniele B. Laucelli³, Bogumil Ulanicki¹, Orazio Giustolisi³

¹*De Montfort University, Leicester, UK, kegong.diao@dmu.ac.uk, (corresponding author)*

²*University of Chieti-Pescara “G. D’Annunzio”, Pescara, Italy*

³*Technical University of Bari, Bari, Italy*

ABSTRACT

Pipe re-sizing of Water Distribution Networks (WDNs) aims at improving the service performance to the required level, while minimizing the cost of replacing pipes in the network. The main challenge comes from the identification of the most effective pipes to re-size from a large number of interacting components. Performing a global search over all pipes in large WDNs does not guarantee to obtain a feasible and efficient solution due to the enormous search space, even by employing advanced algorithms, e.g., evolutionary algorithms. This paper proposes a novel method to reduce the search space for optimal re-sizing based on topological metrics from Complex Network Theory and hydraulic metrics, while providing sub-optimal solutions comparable to the full search solutions, i.e., considering all pipes as candidates. The topological metrics are based on the edge-betweenness tailored for WDN analysis. Hydraulic metrics are unit headloss and flow rates of pipes computed based on simulation of the WDN in the current configuration. The optimal re-sizing plans obtained, particularly that using edge betweenness were tested on a real WDN. The results are comparable with the full search solutions but they are much more efficient to obtain and require replacing mostly contiguous pipes, i.e., easier for practical fieldwork.

Keywords

Optimal pipe re-sizing of water distribution networks, complex network theory metrics, efficient re-sizing plans.

INTRODUCTION

The increase of population in urban areas causes the rise of water demand in water distribution networks (WDNs) while the ageing of pipeline is responsible for the reduction of its hydraulic capacity. The joint effects of these factors progressively increase the risk of insufficient water supply and motivate water companies to increase WDN performance. Since the complete re-building of the system is economically and technically infeasible, effective plans should select only a subset of pipes for rehabilitation, accounting for economic and technical constraints. The rehabilitation commonly focuses on replacement of aged pipes, pipe re-sizing, and pipe duplications. As there is a number of discrete sizes to select for each pipe to rehabilitate, the pipe re-sizing for WDN is a challenging task in water engineering. The task become more difficult when considering the need to identify only the most critical pipes from the enormous number of combinations of interacting pipes. The search for optimal solutions (i.e. set of pipes to be re-sized and the diameters to use) can be formulated as an optimization problem aimed at minimizing the cost of intervention while providing adequate pressure at all demand nodes.

The currently most widely used methods for solving the optimization problem in technical literature are Evolutionary Algorithms (EAs) (e.g., Kim and Mays, 1994; Savic and Walters, 1997; Halhal et al. 1997; Farmani et al. 2005; Giustolisi et al. 2006; Jin et al. 2008; Roshani and Filion, 2015). The key advantage of using optimization models is their ability to consider many decision variables and automatically search potential combinations of re-sizing solutions (Savic and Banyard, 2011).

Although EAs are designed to solve classically NP-Hard problems quite efficiently, they are not computationally efficient especially when applied to real-world large WDNs. Indeed, finding optimal

Pareto front for a WDN with many potential pipes to be re-sized is a major challenge due to the large size of the search space (Kadu et al. 2008). For example, the Exnet WDN (Farmani et al. 2005), which is used as a case study in this paper, has 2465 pipes. If eleven diameter sizes are available as well as a ‘do nothing’ option for pipe replacement, the search space of considering all possible pipe size combinations will be as large as 12^{2465} discrete combinations. Such a huge size of the search space makes many solutions equally good in terms of objective functions, although very different in terms of selected pipes and diameters. Therefore, improving the computational efficiency and quality of solutions, even using EAs, for such optimization problems is needed (Maier et al., 2014).

An emerging way to improve EAs in such kind of problems is to reduce the search space as much as possible. Mala-Jetmarova et al. (2018) classified techniques for search space reduction into algorithm-based and network-based methods. The algorithm-based methods focus on refining the process of searching optimal solutions within the search space, e.g., better strategy of updating some parameters (e.g., the penalty factor) that guides the optimization process. The network-based methods use information resulting from network analysis to directly reduce the size of the search space. Dandy and Engelhardt (2001) placed an upper bound on the number of pipes considered for replacement and/or re-sizing), thus reducing the search space. Kadu et al. (2008) used critical path method (Bhave, 1978) to simplify looped WDNs into a branched system and set lower and upper bounds on the number of candidate sizes for a pipe. However, this may result in a suboptimal solution if candidate pipe sizes are not selected properly. Vairavamoorthy and Ali (2005) used a pipe index vector to measure the relative importance of pipes in a WDN in terms of their impact on the hydraulic performance of the network, and thus excluding regions of the search space where impractical and infeasible solutions exist. Diao et al. (2015) developed a twin hierarchical decomposition technique that simplifies a whole complex WDN to that of smaller subsystems formed by either backbone mains or community feeders. The method enables optimal rehabilitation of all communities independently, without physical division of the WDN.

Muhammed et al. (2017) proposed a clustering-based method for selection of a subset of pipes within the clusters with the lowest pressure and the feeding pipelines between these clusters and the source(s). This paper proposes a search space reduction method to identify a subset of critical pipes for optimal pipe re-sizing of a WDN based on topological and hydraulic metrics. Topological metrics are based on latest advancements in application of Complex Network Theory (CNT) to WDN analysis (Giustolisi et al., 2019). The purpose of the proposed methodology is to provide suboptimal solutions almost as good as full search solutions (considering all pipes as candidates for sizing) but much more efficiently. The reduced size of the search space also minimizes the risk of selecting multiple solutions, which are equally good in terms of objective functions in different runs of the EA, that would put decision-makers in a quandary. From such a perspective, the methodology also improves the technical effectiveness in using EAs for supporting decisions. Specifically, the topological metrics that are used are based on the WDN-tailored edge-betweenness and the hydraulic metrics are based on the unit head loss of pipes and the flow rate. Moreover, the proposed strategy implicitly drives the search towards solutions that select contiguous pipes for re-sizing. This is of practical relevance to planning works since it reduces the number of construction sites and the relevant indirect socio-economic costs.

Therefore, this study aims at introducing a novel strategy to reduce the search space, enabling faster optimization runs, and allowing to compare re-sizing solutions coming from using different metrics. Indeed, the most effective metric to inform such process depends on hydraulic and topological features of the specific WDN.

The next section of the paper summarizes the main formulation of the pipe re-sizing problem as an optimization problem, and the methodology of using metrics (e.g., the WDN tailored edge-betweenness indicators) for search space reduction. The following case study section describes the application of the proposed approach to optimal pipe re-sizing of the Exnet WDN (Farmani et al., 2005, Centre for Water

Systems, 2004). Discussion and conclusions summarize rationales of the proposed procedures, findings and perspectives for future applications.

METHOD

Pipe re-sizing problem

The optimal WDN re-sizing considered in this study aims to meet pressure requirements (i.e. no pressure deficiencies) by replacing some pipes at minimal capital cost. It is formulated as a two-objective optimization model as shown in Eqs. (1)-(5), which takes the minimization of the capital cost, Eq. (1) and the number of demand nodes with pressure deficits into account, Eqs. (2) and (3). Eqs. (4) and (5) represent mass balance and energy conservation in the WDN (implicit constraints), which are automatically fulfilled by simulating the WDN hydraulic model for each candidate solution. It is worth noting that including the minimization of number of demand nodes with pressure deficits as an objective function, rather than a constraint, allows a more effective exploration of the search space, avoiding losing promising solutions during the search.

The formulation in Eq. (4) is quite general and encompasses both, the classic model, based on the demand-driven analysis (DDA), and the advanced model, encompassing pressure-driven formulation of all demand components and leakages (PDA).

$$\text{minimize } Cost = \sum_{k=1}^{np} U_c(D_k, r_k) \cdot L_k \quad (1)$$

$$\text{minimize } NNPD = \sum_{j=1}^{nn} NPD_j \quad (2)$$

$$NPD_j = \begin{cases} 1, & \text{if } P_j < P_{min} \\ 0, & \text{otherwise} \end{cases} \quad (3)$$

s.t.:

$$\sum_j Q_{ij} - d_i = 0, \forall i \in [1, nn] \quad (4)$$

$$H_i - H_j = h_{ij} = Aq_{ij}^B \quad (5)$$

where $U_c(D_k, r_k)$ is the unit cost of pipe k , which is a function of its diameter D_k and the road type r_k . Therefore, the unit cost includes the cost of excavation and repaving, and its value is site-specific. L_k is the length of pipe k ; np is the number of resized pipes in the network. $NNPD$ is the number of nodes with pressure deficit. NPD_j equals 1 if node j shows pressure deficit and zero otherwise. P_j is the pressure of node j , and P_{min} is the minimum pressure required at demand nodes. nn is the number of demand nodes. Q_{ij} is the flow rate connected to node i . d_j is the flow demand at node i and by convention, flow into a node is positive. H_i is the nodal head at node i . h_{ij} is the head loss of the pipe between node i and j . q_{ij} is the flow rate of the pipe between node i and j . A and B are the resistance coefficient and the flow exponent depending on which friction head loss formula is being used.

As mentioned before, assuming n_D decision alternatives, including the do-nothing option, the complete search space of the problem includes n_D^{np} candidate solutions, which means that even in a small WDN with $np = 100$ pipes and 10 commercial diameters, i.e., $n_D = 11$ including the “do nothing” option, the search space encompasses 1.3781×10^{104} alternative solutions.

WDN tailored edge-betweenness to inform pipe re-sizing

The application of metrics borrowed from CNT for evaluating a WDNs' network structure and supporting possible management actions has been extensively explored by Yazdani and Jeffrey (2010, 2011) and Yazdani et al. (2011), i.e., for the analysis of vulnerability and resilience. Edge betweenness, in its original formulation, is a centrality metric of the importance of an edge in a network graph in terms

of connectivity. In general, the edge betweenness is defined as the summation of the fraction of shortest paths that go through the edge in a graph or network (Girvan and Newman, 2002):

$$C_l^B = \sum_{\substack{s \neq t \in V \\ l \in E}} \frac{\sigma_{s,t}(l)}{\sigma_{s,t}} \quad (5)$$

where C_l^B is the edge betweenness of edge l , $\sigma_{s,t}(l)$ is the number of shortest paths from node s to node t passing through edge l , $\sigma_{s,t}$ is the number of all shortest paths from node s to node t . V and E are the sets of nodes and edges in the network. This metric has been tailored for WDN analysis by Giustolisi et al. (2019), by introducing three concepts which are of key relevance for a WDN: (i) embedding the different role of reservoirs, tanks and demand nodes; (ii) introducing appropriate weights for pipes containing the information on the infrastructure; (iii) introducing the prior information on flow direction, i.e., in pipes connected to reservoirs feeding the WDN or with unidirectional devices (e.g., check valves, pressure reduction valves, pumps). Therefore, the WDN-tailored edge betweenness of a single pipe represents the number of the shortest paths that go through the pipe. Weights are interpreted as distances and used to calculate weighted shortest paths, i.e. the length of the path is decided by the sum of the weights of all the pipes on the path. The information on flow direction enables identification of the shortest paths based on not only the topological connectivity but also the flow conditions of the hydraulic devices.

Giustolisi et al. (2019) demonstrated that all such modifications enable an effective analysis of the key features of the physical domain of the WDN (i.e., the network), which captures the emerging hydraulic behaviours of the system, even before running the hydraulic simulation. Indeed, domain analysis through WDN-tailored CNT metrics helps understanding in advance the role of pipes in determining the system hydraulics independently of the specific set of boundary conditions (e.g., demands, water level in tanks, etc.). Nonetheless, the same authors discussed the usage of different edge weights to pipes for different purpose of analysis.

The present work exploits the idea to use the WDN-tailored edge betweenness to inform the identification of a subset of pipes that are more effective than others for the re-sizing. It consists of the two steps, that need to be performed before running the optimization routine (e.g., using EAs):

- 1) WDN-tailored edge betweenness of every pipe element is computed by accounting for weights representing features of the pipes;
- 2) A subset of pipes with the highest values of the WDN-tailored edge betweenness is selected as a candidate for re-sizing.

Since this strategy aims at supporting pipe re-sizing, three kinds of weights are analysed in the present work:

- 1) “connectivity”: all pipes are unweighted in order to account for information on connectivity only, meaning that the information on topological connectivity described by the edge-betweenness in Eq. (5) identifies the most relevant pipes;
- 2) “resistance”: each pipe is assigned a weight equal to $L_k \cdot D_k^{-5}$, which is proportional to the hydraulic resistance per unit flow through the pipe, according to Darcy-Weisbach equation; meaning that the pipes with higher hydraulic resistance are the most effective for re-sizing;
- 3) “length”: each pipe is assigned a weight equal to L_k , meaning that longer pipes affect more WDN hydraulics and background leakages.

It is worth noting that the weight representing “resistance” is formulated to use pipe features that are commonly available (i.e., pipe length L_k and diameter D_k) and do not depend on uncertain internal roughness conditions and hydraulic variables (i.e., flow regime).

For the sake of discussion in this paper, the WDN-tailored edge betweenness in each of the previous weighting strategies is computed accounting (“direct”) or not accounting (“indirect”) for the information on flow direction (i.e., flow from reservoirs, unidirectional devices). This means that six possible

rankings of pipes are obtained by using various WDN-tailored edge betweenness metrics: “connectivity-direct”; “connectivity-indirect”; “resistance-direct”; “resistance-indirect”; “length-direct”; “length-indirect”.

WDN hydraulic metrics to inform pipe re-sizing

The information on WDN hydraulics is already available from the analysis of the system as is, possibly with pressure deficit at some nodes. Such observation motivated the idea to inform the reduction of the search space by ranking pipes according to the two main hydraulic features, retrieved from hydraulic simulation, as described below:

- 1) “Pipe flow”: it comes from the assumption that pipes carrying the largest flow are the main “arteries” of the system, which drive the main WDN hydraulic behaviour and are supposed to be the most effective for re-sizing.
- 2) “Unit head-loss”: the pipes with the highest head-losses per unit length can be seen as responsible for insufficient pressure at water demand nodes; therefore, their replacement is potentially effective to solve local pressure deficit conditions.

Also, in the present work a fixed percentage of pipes (30% of pipes with the highest hydraulic metrics) is identified as a subset of candidate pipes for re-sizing, although such selection can be performed using different criteria.

Search space reduction based on the metrics and clustering

To facilitate the search space reduction (i.e., select a subset of pipes as candidates for re-sizing), the WDN is first divided into clusters by using the modularity-based clustering. Details of the method is available in Diao et al., 2012 and 2014. Pipes in all clusters are ranked in descending order of the metric used (e.g., pipe flow). Next, a percentage of pipes (e.g., 30%) with the highest values of the metric are selected as candidate pipes (this work assumes a fixed percentage of pipes over all metrics although this

can be performed in various ways, e.g., based on total maximum length). In the same way, the selection is made for pipes connecting clusters. From optimization perspective, this aims to reduce the size of the search space from n_D^p to $n_D^{(0.3 \times p)}$; assuming $n_D = 11$ and $n_p = 100$, the search space reduces to about 1.74×10^{31} , about 73 orders of magnitude smaller than the full search space. Therefore, the complete set of candidate pipes includes both selected inner-cluster and inter-cluster pipes.

Please note that the decision on the percentage of the selected pipes depends on the WDN and might follow various approaches. Using a large percentage will make the constraints easier to satisfy but it also means a larger search space and vice versa. The methodology used herein to identify the percentage is defined using the following procedure: 1) the mean of the values of each metric are calculated; 2) the percentage of pipes that have their metric values higher than or equal to the mean is calculated for each metric; 3) the largest value of these percentages among all the metrics is identified (e.g. 24%); 4) a value slightly higher than the largest percentage identified in step 3 is used (e.g. 30%) for every metric-informed optimization to make the analyses comparable.

CASE STUDY: EXNET WDN

The Exnet WDN model was set up by the Centre for Water Systems (CWS) at the University of Exeter as a realistic and challenging rehabilitation problem (Farmani et al. 2005). The use of the Exnet WDN enables to focus on the proposed methodology to reduce the search space, while ignoring some other aspects related to real systems such as background leakages and pressure-driven demands.

The network serves a population of approximately 400,000 and was supposed to be reinforced to meet the projected demand in 2020. It consists of a great number of pipes with a wide head loss range. The layout of Exnet is shown in Figure 1, with highlighted 16 clusters obtained by using the modularity method proposed in the section above. It contains 2,465 pipes, 5 valves, 1,891 junction nodes and 7 water sources. Two major reservoirs (node 3001 and 3002) supply water to the system at fixed heads of 58.4

m and 62.4 m respectively. The system is also fed by its neighbouring systems via nodes 3003 to 3007 at fixed rates. Three non-return valves (also known as check valves) are connected to node 3001 and 3002 to control the flow direction into and out of the system. One pressure reducing valve is in the downstream of node 3004 to maintain the downstream pressure below 58.4 m. One throttle control valve is also in the link downstream of node 3004 to control the flow and pressure of the system. The minimum pressure requirement for demand nodes is set to 15 m according to Muhammed et al. (2017).

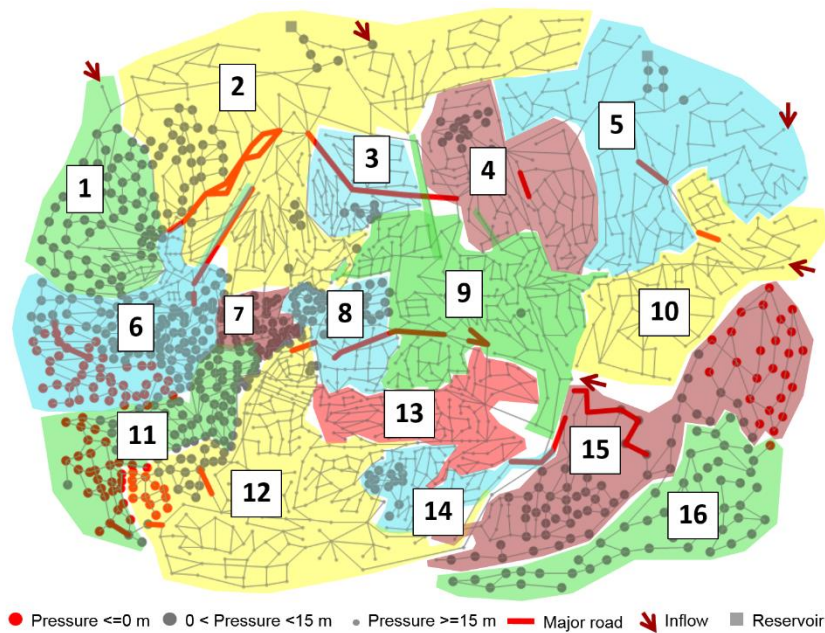


Figure 1. Layout of the Exnet model with nodal pressure and clustering information

The re-sizing options for Exnet are restricted to eleven discrete pipe sizes available for replacement (Table 1) and one extra option of 'do nothing'. Therefore, from the mathematical perspective, the complete search space is equal to 12^{2465} discrete combinations, which makes it an extremely large and complex combinatorial optimization problem. Please note that a few changes were made in the original EPANET2 input file of Exnet. All the changes are listed in Table 2 and the new input file for this case study is provided as supplementary data. The changes were made to neglect those devices that would bias the hydraulic functioning of the re-sized WDN. In addition, replacing the inflows (i.e., negative

water demands at five junctions) with reservoirs at the same nodes, aims at keeping the direction of inflow pipes consistent with the original network feeding scheme.

Table 1. Commercial pipe sizes and costs for the Exnet case study

Diameter	Roughness [Darcy-Weisbach]	Cost/m [€]
new DN 100	0.003713306	158.49
new DN 150	0.006074969	234.98
new DN 180	0.006832484	258.14
new DN 200	0.007589998	280.79
new DN 250	0.009565477	337.87
new DN 300	0.010783442	371.87
new DN 400	0.013672394	449.64
new DN 500	0.018566532	574.35
new DN 750	0.027849798	794.42
new DN 1000	0.037133064	1000
new DN 1200	0.044559677	1157.03

Table 2. Summary of changes made in the original Exnet.inp file: pipes and junctions

PIPES
<i>Deleted PRV</i>
- pipe ID with PRV: prv
- ID of controlled node 120
<i>Deleted Minor loss coefficient</i>
- pipe ID with Minor loss: 1919
<i>Check valve changed into closed pipe</i>
- pipe ID of the closed pipe: 4177
<i>Deleted Check valve</i>
- pipe ID with deleted check valve: 5309
NODES (Junctions)
<i>Pressure for correct service:</i>
- 15m for all nodes with non-null demands
- 5m for node ID: 1107

Table 3. Summary of the changes made in the original Exnet.inp file: nodes with inflows changed into reservoirs

Node ID in INP	Elevation (m)	Nodal Pressure before changes (m)	Inflows (m³/s)	New Reservoir Head (m)
-----------------------	--------------------------	--	--------------------------------------	---------------------------------------

3003	11	31.74	-0.063	42.74
3004	73.54	-5.88	-1.388	67.66
3005	30.5	35.53	-0.0107797	66.03
3006	33	34.65	-0.9260001	67.65
3007	16	26.75	-0.0261027	42.75

The EA adopted in this work is the NSGAI (Deb et al., 2002) algorithm, although any other optimization method can be used without influencing the effectiveness of the proposed strategy. For optimal re-sizing, the NSGAI was independently run five times to alleviate the impact of randomness on the solutions obtained. Each re-sizing solution was evaluated by invoking the EPANET2 solver (Rossman, 2000), which implements classical demand-driven analysis. On one hand this choice was due to the lack of information on water losses, which prevented from calibrating an advanced pressure-driven model see the last equation among Eq. (3). At the same time, using a classical model keeps the main focus of this work on the proposed strategy for reducing the search space, avoiding possible bias due to pressure-driven formulation of demands and leakages.

The total number of function evaluations (NFEs) was set to 10 million for all the optimization runs, in order to make results independent of the number of iterations. The population size was fixed at 50, with 200,000 generations evolving during the optimization process. For each pipes selection strategy the 30% of pipes with the highest values of each metric was selected as candidate pipes. This is because the percentage of pipes with their metric values higher than or equal to the mean is between 20%-25%. The optimization was repeated five times; for each time and each pipes selection strategy, only the optimal solutions with $NNPD=0$ are considered as feasible. The average cost of the solutions among the five runs is selected for numerical comparison (Figure 2), while the minimum cost solution is selected for topological evaluation (Figure 3).

RESULTS AND DISCUSSIONS

Figure 2 compares the average cost (i.e., computed in the five-optimization runs as above-mentioned) of each re-sizing solution for the Exnet WDN based on the eight different strategies described while Figure 3 details each of the minimum cost solution's topology showing the candidate pipes for re-sizing in red. In the far right of Figure 2, in dark-blue bar the mean cost (20.9 M€) of re-sizing plans using full-search is displayed. This kind of strategy has highly variable solution costs, ranging from 7.4 M€ to 32.5 M€, obtained over multiple runs of the full space search. This high variability is probably due to the dependence on the initial conditions of the NSGAI optimizer (Deb et al., 2002) and on the size of the search space, so it can be said that as the size and the complexity of a WDN increases, the variability can get wider.

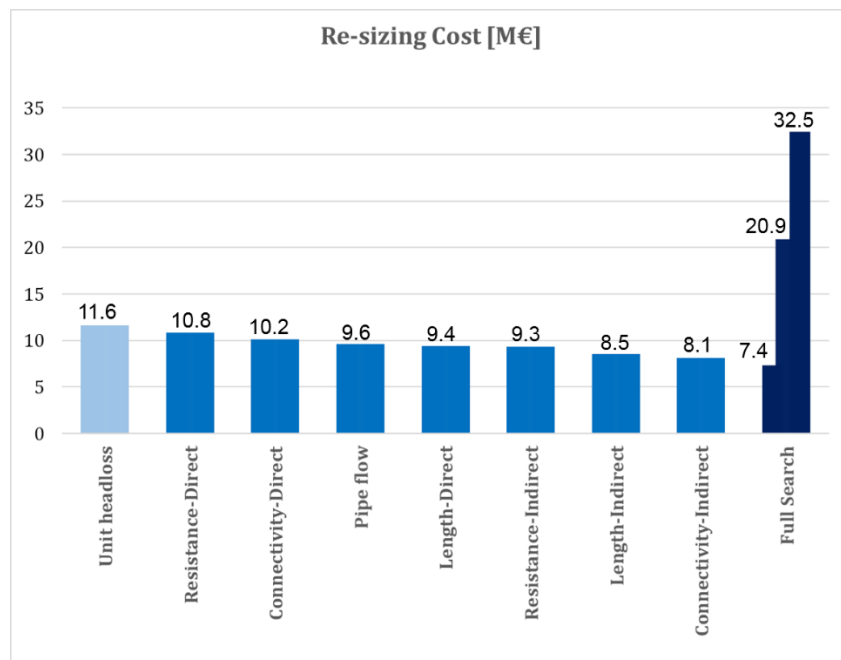


Figure 2. Average costs of re-sizing solutions of Exnet for various search strategies.

Figure 3(A) shows the topology of the cheapest solution (7.4 M€) obtained from the full space search. It can be noted that the EA selected pipes are quite spread over the entire WDN, with many small pipes

separated from the others. This observation means that, although such solution would solve pressure deficit conditions, it requires several construction sites to carry on works, which is not effective from practical standpoint.

The “pipe flow” case (Figure 3(B)), shows many pipes on the same water path that are connected with each other for re-sizing, thus providing a plan to be easier implemented in practice. However, the pipes selected are those with highest flow rate; therefore, those pipes have larger diameters and replacing them is expected to be more expensive. Results are consistent with the fact that the solution cost is about 9.6 M€. As for the topological metrics informed solutions (Figures 3(C) and 3(D)), it can be noted that the “direct” cases are more expensive (10.2 M€) than their “indirect” counterparts (8.1 M€). In fact, “direct” metrics account for directional devices and for known pipe flow directions (i.e., from reservoirs to the network), allowing shortest paths in one direction only. This, in turns, results in reduced value of the centrality metrics, i.e., the edge betweenness in the ‘direct’ case, it only applies to the pipes where only one direction is allowed. Therefore, some pipes in the highest 30% of edge betweenness that are in the ‘indirect’ case, are not in the “direct” case and cannot be selected as candidate for re-sizing. Such pipes usually are not that long (e.g., pipes close to reservoirs); this means that their re-sizing potentially allows considerable improvement in pressure with lower cost.

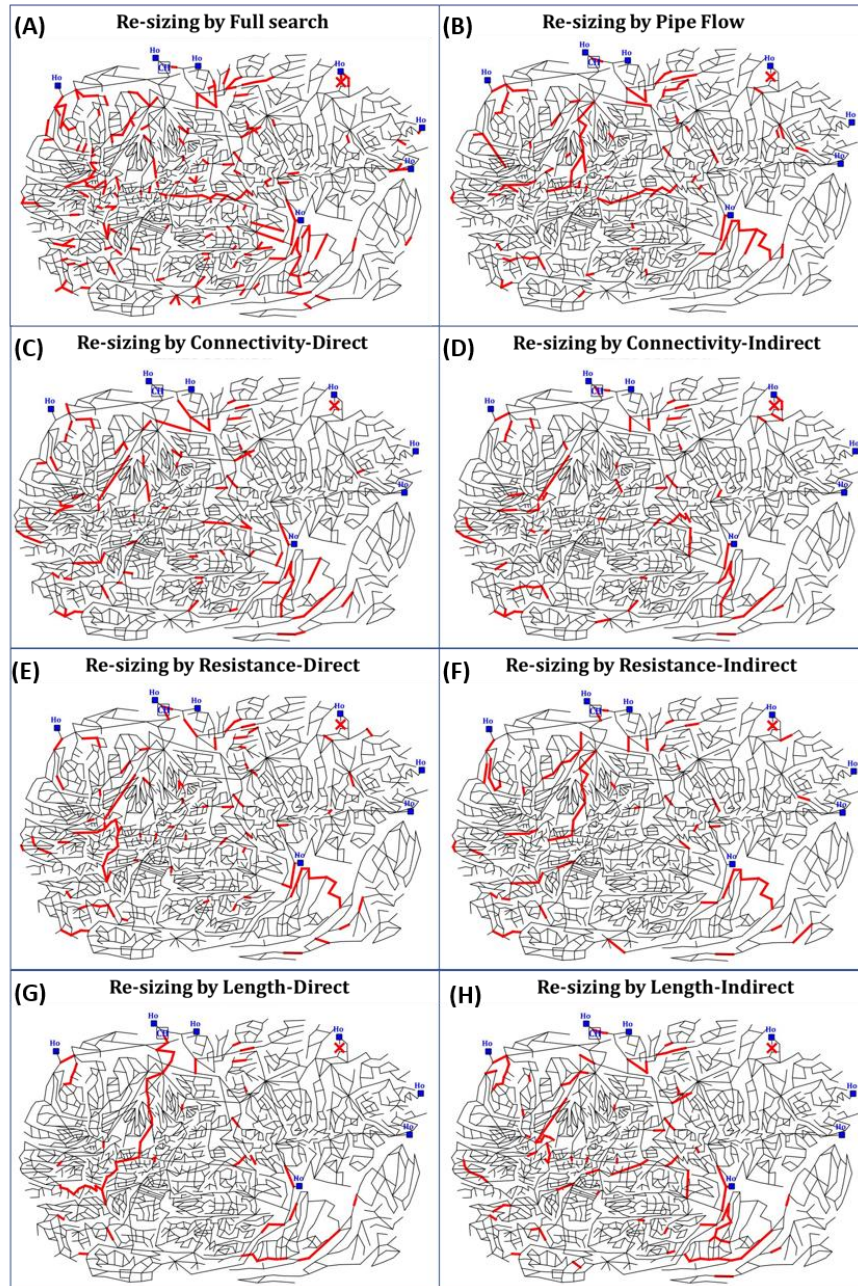


Figure 3. Re-sizing solutions topologies of Exnet for various search strategies

It is worth noting that the selection of pipes based on the highest edge betweenness, without using any weights, results into the cheapest solution among all search space reduction strategies used in this study. This is likely due to the circumstance that, in the case of the Exnet WDN, the connectivity information is quite useful to determine the most effective water paths. As such, the topological informed search

drives the EA to explore solutions into a subset of pipes whose re-sizing has effective impact on WDN hydraulic behaviour.

The cases of “resistance-indirect” and “length-direct” are shown in Figure 3(F) and 3(G), which are those with the lowest dispersed pipes in the WDN, and many contiguous pipes (i.e., pipes on the same path). Many pipes also coincide with those selected in the “pipe flow” case, demonstrating that a close relationship exists between WDN topology domain and hydraulics (Giustolisi et al., 2019).

The outcome that using the tailored edge betweenness with the weights of “length” provides better results than those with “resistance” can be explained considering that longer pipes usually carry water from different regions of a WDN, thus having more impact on WDN hydraulic behaviour (i.e., sufficient pressure at nodes). Vice versa, the hydraulic resistance increases as diameter decreases, and smaller diameters are usually in the looped (distribution) part of the WDN, which have only local impact on pressure; therefore, getting sufficient pressure at all nodes may require the replacement of a larger number of pipes providing more expensive solutions.

Such observation hints that the results of various optimization runs based on different metrics to reduce the search space, may provide a ranking of pipes that are the most effective to solve pressure deficit conditions due to their topological position and hydraulic asset features. Such pipes can be included as *a priori* in further optimization runs.

CONCLUSIONS

This work proposes a strategy to reduce the search space for the optimal re-sizing problem, informed by hydraulic and topological metrics. The latter ones come from the latest advancements in centrality metrics tailored for WDN from Complex Network Theory (CNT). Specifically, the pipe re-sizing optimization uses a subset of pipes selected based on the highest values of metrics entailing topological

and hydraulic features rather than all pipes. The strategy described above was proposed as a support to the planning of the re-sizing of a real WDN in the presence of insufficiencies of the user supply service. In particular, the analyses of the different metrics is aimed at evaluating their effectiveness within the proposed strategy, with the final purpose of making its application flexible with respect to the specificities of existing real networks. The goodness of the previously described results allows us to foresee its potential application in the “real life” of a municipality which, based on the information and knowledge available, will be able to adapt the search to the characteristics of the considered WDN.

The procedure was tested on a large WDN, known benchmark in the literature, and the results show that the strategy is performing well, providing cheaper solutions if compared with the average cost got from multiple runs of the same optimization algorithm on the complete search space (i.e., taking all pipes as candidate for re-sizing). It is notable that most of the re-sizing plans obtained from applying the search space reduction strategy reports contiguous pipes. This is a positive aspect for an effective support to planning interventions on the WDN, because by reducing the number of construction sites if compared with the solutions found in the full search case, it reduces direct and indirect intervention costs (e.g., nuisance to traffic and to the city life in general). It must remark that having contiguous pipes in the reduced search space is a direct consequence of using topologically informed metrics, which is likely to return similar (high) values of edge betweenness in pipes close to each other.

Furthermore, the case study shows that not accounting for flow directions (i.e., “indirect” case) in edge betweenness calculation performs better than other metrics. This is because the “direct” metrics account for shortest paths in one direction only, and thus ignored some pipes playing an important role in topological connectivity regardless of flow directions.

The idea of using WDN-tailored metrics from CNT, that is presented in this work opens new directions for future research. About optimization strategies, various topological metrics can be used in sequence during the same optimization run, e.g., progressively reducing the search space, or in conjunction, i.e.,

to select a subset of pipes with the highest values of different metrics. Future studies could also explore other metrics, not reported here for reducing the search space, combining information on network topology and relevance of nodes (e.g., Giustolisi et al., 2020). Finally, the same approach could be tested on multi-objective optimization search, embedding additional objective functions related to different technical aspects (i.e., pipe deterioration, pipe material information, number of private connections, etc.). There are two main limitations in the results presented. First, the simplified assumption on hydraulic model (i.e., EPANET2) used in the case study does not account for pressure-dependent leakages and users' demand. Although this makes the WDN re-sizing problem close to the classical WDN design problem, this assumption allows keeping the focus on the impact of the proposed strategy on optimization. Nonetheless, future works can explore the effectiveness of the approach proposed here on real complex WDNs employing an advanced hydraulic modelling. Secondly, for the pipes selection strategy, the decision on the percentage of the selected pipes depends on the analyzed WDN and some techniques can be investigated to automatically decide the optimal percentage regardless of the metric applied for pipes selection.

REFERENCES

- Barthelemy, M. (2014). Discussion: Social and spatial networks. *Les Nouvelles de l'Archéologie*, (135), 51–61. <https://doi.org/10.4000/nda.2374>
- Barthélemy, M., and Flammini, A. (2008). Modeling urban street patterns. *Physical Review Letters*, 100(13), 138702. <https://doi.org/10.1103/PhysRevLett.100.138702>
- Centre for Water Systems. (2004). *EXNET water distribution network*. <http://emps.exeter.ac.uk/media/universityofexeter/emps/research/cws/downloads/exnet.inp>
[Accessed on 30th January 2022].

- Dandy, G.C.; Engelhardt, M. (2001). Optimal scheduling of water pipe replacement using genetic algorithms. *J. Water Resour. Plan. Manag.* ASCE, 127, 214–223.
- Deb, K., Pratap, A., Agarwal, S. and Meyarivan, T. (2002). A fast and elitist multiobjective genetic algorithm: NSGA-II. *IEEE Transactions on Evolutionary Computation.* 6(2), 182. doi:10.1109/4235.996017.
- Diao, K., Zhou, Y. and Rauch, W. (2012). Automated creation of district metered area boundaries in water distribution systems. *J. Water Resour. Plan. Manag.* ASCE, 139(2), 184-190.
- Diao, K.G., Farmani, R., Fu, G.T., Astaraie-Imani, M., Ward S. and Butler, D. (2014). Clustering analysis of water distribution systems: identifying critical components and community impacts, *Water Sci. Technol.*, 70(11), 1764–1773.
- Diao, K.G., Fu, G.T., Farmani, R., Guidolin, M. and Butler D. (2015). Twin Hierarchy Decomposition for Optimal Design of Water Distribution Systems. *Journal of Water Resources Planning and Management*, C4015008.
- Engelhardt, M.O., Skipworth, P.J., Savic, D.A., Saul, A.J. and Walters, G.A. (2000). Rehabilitation strategies for water distribution networks: A literature review with a UK perspective. *Urban Water*, 2, 153–170.
- Farmani, R., Savic, D.A. and Walters, G.A. (2005). Evolutionary multi-objective optimization in water distribution network design, *Engineering Optimization*, 37 (2), 167-183.
- Giustolisi, O., Laucelli, D. and Savic, D. A. (2006). Development of rehabilitation plans for water mains replacement considering risk and cost-benefit assessment. *Civil Eng. and Environ. Syst.*, 23(3), 175-190.
- Giustolisi, O., and Walski, T. (2012). Demand components in water distribution network analysis. *Journal of Water Resources Planning and Management*, 138(4), 356–367.

- Giustolisi, O., Simone, A., and Ridolfi, L. (2017). Network structure classification and features of water distribution systems. *Water Resources Research*, **53**, 3407–3423.
- Giustolisi, O., Ridolfi, L., and Simone, A. (2019). Tailoring centrality metrics for water distribution networks, *Water Resources Research*, **55**(3), 2348-2369.
- Giustolisi O., Ridolfi L. and Simone A. (2020). Embedding the intrinsic relevance of vertices in network analysis: the case of centrality metrics. *Nature Scientific Report*, **10**, 3297.
- Halhal, D., Walters, G.A., Ouazar, D., Savic, D.A. (1997). Water network rehabilitation with structured messy genetic algorithm. *J. Water Resour. Plan. Manag.*, **123**, 137–146.
- Lansey, K.E.; Mays, L.W. (1989). Optimization model for water distribution system design. *J. Hydraul. Eng.*, **115**, 1401–1418.
- Jin, X., Zhang, J., Gao, J.-L., Wu, W.Y. (2008). Multi-objective optimization of water supply network rehabilitation with non-dominated sorting genetic algorithm-II. *J. Zhejiang Univ. Sci. A*, **9**, 391-400.
- Kadu, M.S., Gupta, R., and Bhave, P.R. (2008). Optimal design of water networks using a modified genetic algorithm with reduction in search space. *J. Water Resour. Plan. Manag.*, **134**, 147–160.
- Maier, H.R., Kapelan, Z., Kasprzyk, J., Kollat, J., Matott, L.S., Cunha, M.C., Dandy, G.C., Gibbs, M.S., Keedwell, E., Marchi, A., Ostfeld, A., Savic, D., Solomatine, D.P., Vrugt, J.A., Zecchin, A.C., Minsker, B.S., Barbour, E.J., Kuczera, G., Pasha, F., Castelletti, A., Giuliani, M., Reed, P.M., 2014. Evolutionary algorithms and other metaheuristics in water resources: current status, research challenges and future directions. *Environ. Modell. Software* **62**, 271–299.

- Muhammed, K., Farmani, R., Behzadian, K., Diao, K., and Butler, D. (2017). Optimal rehabilitation of water distribution systems using a cluster-based technique. *J. Water Resour. Plan. Manag.*, **143**.
- Roshani, E. and Filion, Y. (2015). Water distribution system rehabilitation under climate change mitigation scenarios in Canada. *J. Water Resour. Plan. Manag.*, **141**.
- Savic, D.A. and Banyard, J. (2011). *Water distribution systems*. 1st ed. London: ICE Pub.
- Savic, D.A. and Walters, G.A. (1997). Genetic algorithms for least-cost design of water distribution networks. *J. Water Resour. Plan. Manag.*, **123**, 67–77.
- Vairavamoorthy, K., Ali, M. (2005). Pipe index vector: A method to improve genetic-algorithm-based pipe optimization. *J. Hydraul. Eng.*, **131**, 1117–1125.

Data Availability

The EPANET input file of the case study water distribution network, that include all modifications, is available at: https://www.researchgate.net/publication/359109267_exnet_EPS_mod